

Subject: Levee Vulnerability Assessment (Final Draft)

**Project feature:** Projectwide

Prepared for: California Department of Water Resources (DWR) / Delta Conveyance Office (DCO)

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# 1. Purpose, Introduction and Background

The Delta Conveyance Project (Project) would include intakes along the Sacramento River between the confluences with American River and Sutter Slough and a tunnel between the intakes and a forebay at the downstream terminus of the main tunnel referred to as the Southern Forebay. Water would either flow by gravity or be lifted by the South Delta Pumping Plant from the tunnel into the Southern Forebay. Discharge from the Southern Forebay would occur through the Southern Forebay Outlet Structure, at the south end of the reservoir, into the South Delta Conveyance Facilities for connection to the existing State Water Project Harvey O. Banks Pumping Plant and Central Valley Project C.W. Bill Jones Pumping Plant (Jones).

Two alignments are considered in this technical memorandum (TM), the Central and Eastern corridors, along with a varied range of flow capacities. The internal diameter of the tunnels would vary depending on the project design flow capacity but is anticipated to range between 26 feet and 40 feet. The corridors would be located primarily within the Delta, which is a flood-prone area with many islands and tracts below sea level and protected by approximately 1,100 miles of earthen levees (Arcadis, 2017). In most cases, levees were constructed over many years to protect the island interiors from inundation and reduce flood risk to the residents and workers operating within the Delta (DWR, 1982). The ability for these levees to withstand hydraulic loading is critical to the success of the Project; many of the levees will be relied upon to provide flood protection during construction as well as protect permanent assets and infrastructure. Additionally, many levees within the Delta have roads constructed on their crowns which provide regional access within the Delta. It is an objective of the Project to avoid to the extent practical the use of existing levee crowns as haul routes, but existing roads are still critical for everyday traffic, levee maintenance and monitoring, as well as access to some Project locations. Conceptual development of Project options and features warrants consideration of the condition of existing levees given the importance of flood protection in the Delta and the significant role of levees in the Delta.

### The purpose of this TM is to:

- Provide an overview of existing levee geometry standards
- Describe the approach and findings of a relative vulnerability assessment of levees within the proposed tunnel corridors

# 1.1 Summary of Results

A brief summary of the results from this study are presented below.

- Five criteria and corresponding numeric scoring system were developed to assess relative levee vulnerability which include:
  - Criterion 1 Levees Meeting Levee Geometry Standards
  - Criterion 2 Freeboard Against the 100-year Flood Elevation
  - Criterion 3 Ditches Proximity (if present) to landside toe of levee or berm
  - Criterion 4 Vulnerability to Sea Level Rise
  - Criterion 5 Change in Levee Crest Elevation Between 2007 and 2017 LiDAR
- Each relative vulnerability criterion is evaluated by cross-section and is summarized as the percentage of levee cross-sections evaluated within a levee system meeting a specific standard established by the criterion. The individual vulnerability criteria are weighted and combined to provide a relative vulnerability score by cross-section. The vulnerability scores of all cross-sections are grouped and sorted into quartiles and assigned a levee rating of "Very Low," "Low," "Medium," or "High," relative vulnerability.
- A summary of key statistics that can be extracted from the vulnerability criteria when considering the entire data set used in this assessment includes:
  - Criterion 1 72% of levee cross-sections reviewed have geometry that meet or exceed the Public Law 84-99 (PL84-99) Delta-specific levee geometry
  - Criterion 2 86% of levee cross-sections reviewed have at least 1.5 feet of freeboard above the 100-year flood elevation level
  - Criterion 3 44% of levee cross-sections reviewed do not have a toe-ditch, or if a toe-ditch is present, it is beyond 4 levee heights from the landside levee toe or beyond 2 levee heights from the berm toe
  - Criterion 4 47% of levee cross-sections reviewed have at least 1.5 feet of freeboard above an 100-year flood elevation that considers potential Sea Level Rise (SLR)
  - Criterion 5 82% of levee cross-sections reviewed have a 2017 crest elevation that is within
     0.5 feet of the 2007 crest elevation

# 2. Methodology

The levees in the Delta are exposed to many hazards that may damage or cause failure, resulting in flooding of the island interior. The most significant hazards are due to hydrologic, hydraulic, and seismic (earthquake) loading which can lead to seepage and stability-related failures. A variety of site-specific conditions can also contribute to a levee's vulnerability for failure when subjected to loading including poor/weak embankment or foundation soils, insufficient levee geometry (height, width, and slope inclination), and various types of particularly damaging animal activity or vegetation growth.

The goal of this assessment is to evaluate indicators of levee condition that are not heavily reliant on site-specific subsurface data while providing meaningful results to compare levee vulnerability. Existing levee geometry can provide an indication of how levee systems may perform during different loading conditions and can provide an even stronger indication of how levees might perform relative to one another. Larger levees that are tall with wide crests and shallow slopes will inherently be less vulnerable compared to smaller levees with similar composition, loading and foundation conditions. Important

geometric considerations related to levee vulnerability which can be extracted from topographic data include:

- Overall levee cross-sectional geometry (levee height and slope inclinations) which inherently provide
  a metric of seepage and slope instability susceptibility
- Freeboard which provides a direct measure of the maximum flood level a levee can protect against which translates to a risk of overtopping
- Proximity of a toe-ditch to the levee toe (if present) which may thin or penetrate subsurface fine-grained blanket layers and increase underseepage and slope instability susceptibility
- Vulnerability to SLR which evaluates the current condition of levees under increasing future water levels.
- Past changes in levee crest elevation provides an indication of potential future levee settlement and, in turn, reflect areas that may require future levee modifications to maintain flood protection

Criterion specific to each of the above considerations was developed to evaluate levees within each corridor. The evaluation was performed using cross-sections developed every 500 feet along the levee alignments using LiDAR data collected and provided by the Department of Water Resources (DWR). The geometric criteria developed for this study do not provide a comprehensive evaluation of a levee system or guarantee levee performance. The results provided herein are intended to help locate project infrastructure and better understand potential flood risks within the DWR Notice of Preparation (NOP) corridors (DWR, 2020). This vulnerability assessment does not replace the need for site specific investigations, testing, and analyses. This initial screening-level levee relative vulnerability analysis involved three primary steps:

- Develop relative vulnerability criteria
- Apply criteria to levee cross-sections throughout the project area
- Summarize frequency of vulnerability rankings by levee system

## 2.1 Information Sources

### 2.1.1 Background

Numerous programs and supporting studies have been performed to evaluate the conditions of existing levees in the Delta and potential vulnerabilities due to flooding and seismic events. The programs included assessments on the Delta's ecosystem and habitat, water quality, water availability, natural hazards impact, land use, economic impact, etc. The levee studies were performed with various objectives and levels of detail and culminated in providing a range of data compilations and levee performance evaluations with varying approaches and spatial distributions. Specific programs that contained information used to support the screening-level levee vulnerability assessment discussed herein include:

Delta Risk Management Strategy (DRMS) 2005-2008: The intent of the program was to evaluate the
performance of the levees under various stressors and hazards and assess the potential consequences
(risks) for economic, public health, and safety in the event of levee failures. The DRMS study was
performed in two phases, Phase 1-Risk Analysis, and Phase 2-Risk Reduction and Risk Management
Strategies. The source information pulled from the Delta Risk Management Strategy (URS, 2008) used
for this effort is presented in Section 2.1.2.

Other studies reviewed that contain pertinent site-specific information that could support design level engineering, but were not incorporated into this screening level assessment are:

- Delta Levee Investment Strategy (DLIS) 2013-2017: The DLIS study area included the Sacramento-San Joaquin Delta and the Suisun Marsh. The program applied a risk analysis methodology to identify an investment strategy for the State based on risks to the levee system by considering threats to the levees and the assets they protect. The DLIS study evaluated the probability of flooding for a given island/tract considering two hazards that could cause a levee to breach; hydraulic flooding from high water or seepage, and seismic activity. The vulnerability to each of these hazards was represented with a fragility curve and the condition of the levee.
- Central Valley Flood Protection Plan (CVFPP) 2008-2012: The intent of the CVFPP is to provide a
  systemwide plan to manage flood control facilities that are part of the State Plan of Flood Control
  (SPFC). SPFC includes projects and facilities that the Central Valley Flood Protection Board and DWR
  have provided assurance of cooperation to the federal government. Non-Project levees are not within
  the plan.
- Urban Levee Evaluation Program (ULE) 2008-2014: The purpose of the Urban Levee Evaluation (ULE) Program was to evaluate urban Project levees and appurtenant Non-Project levees and determine whether they meet established United States Army Corps of Engineers (USACE) levee design criteria for seepage and slope stability. The program also identified potential levee deficiencies with recommended improvements, repair alternatives, and associated costs. The evaluation process was performed in several steps, beginning with historical data collection and preparation of a Technical Review Memorandum (TRM). Additional subsurface geotechnical and laboratory testing was then performed and summarized in a Geotechnical Data Report (GDR). Based on the findings from the TRM and GDR, a Geotechnical Evaluation Report (GER) was prepared that presented seepage, slope stability, and seismic evaluations. The GER also included analyses and recommendations for remediation of those levee segments not meeting criteria. The ULE program divided the SPFC system into urban "Study Areas" for investigation, analysis, and reporting. Study Areas that have potential relevance to the Project include the Sacramento River, West Sacramento, and South West Sacramento Study Areas.
- Non-Urban Levee Evaluation Program (NULE) 2008-2014: The purpose of the Non-Urban Levee Evaluation (NULE) Program was to evaluate non-urban Project levees and appurtenant Non-Project levees and determine whether they meet a defined geotechnical design criteria. The program covered a large geographical area, and as such, was divided into two "basin" study areas; the North NULE study area encompasses the area within the Sacramento River Basin, and the South NULE study area encompasses the area with the San Joaquin River Basin. The evaluation of the levees within each study area was performed in two phases. Phase 1 evaluation entailed using non-intrusive studies using readily available information and preparing a Geotechnical Assessment Report (GAR). The GAR included an assessment of each levee segment and categorized the segment based on a determined hazard level. Phase 2 evaluation entailed performing field and laboratory testing, geotechnical analyses, and preparing a GDR and a corresponding Geotechnical Overview Report (GOR) for specific areas or segments.
- California Bay-Delta Program (CALFED) 1994-2008: The intent of the CALFED Program was to systematically address the concerns and reliability of the Delta water supply through the participation and coordination with federal, State, and local governments. There were four main areas that the CALFED Program focused on: Ecosystem Health, Water Quality, Water Supply Reliability, and Levee System Integrity. The intent of the Levee System Integrity focus area, or CALFED Levee Program, was to identify and reconstruct those levees not meeting USACE PL84-99 Delta Specific Standard. One of the premises is that the system-wide levee stability is improved if all levees meet this standard.

 Available Reclamation Districts 5-year plans: The purpose of the 5-year plans is for individual reclamation districts to summarize the engineering, construction and funding goals needed to meet or maintain a specific levee standard to increase flood protection to qualify for state and/or federal funding.

### 2.1.2 Data Sources

Data used for this levee vulnerability assessment includes:

- Flood Elevation Data: The 100-year flood elevations used for the assessment are based on geographic information system (GIS) data compiled by DWR for *Analysis of Delta Levees Compliance of HMP [Hazard Mitigation Plan] and PL 84-99 Design Geometry* (DWR, 2011) as described in the DLIS. The hydrologic inputs are largely based on previous hydrology studies prepared by USACE in 1976 and 1992 for the Sacramento San Joaquin Delta (USACE, 1976; 1992). The Bulletin 192-82 theoretical prism check is the only component of this relative levee vulnerability evaluation that relies on the 300-year flood elevation data. The 300-year flood elevation was not available in GIS format from DWR, so the 300-year flood elevations by levee system were hand-estimated using the hydraulic profiles provided in the USACE 1992 study. Locations not included in the USACE 1992 study were assumed to have a 300-year flood elevation 0.5 feet above the 100-year values provided in the DWR GIS data based on the average difference observed on existing profiles throughout the Delta.
- Levee Stationing: Levee stationing and levee alignments within the study area are based on data
  accumulated by the DWR Delta Levees Program, which has been updated from time to time based on
  updated levee alignment information provided by local levee reclamation districts and maintenance
  agencies. The fundamental source of the levee stationing was provided as station points in the USACE
  1992 study.
- Organics/Peat Thickness: The thickness of organic material in the levee foundation as an input to
  define the required landside slope geometry to meet Delta levee geometry standards. Contours of
  organics/peat thickness were previously developed and digitized for the Delta Risk Management
  Strategy (URS, 2008) and were used in this levee vulnerability assessment.
- LiDAR Survey Data: The cross-sectional data used in this vulnerability assessment was developed using LiDAR data obtained by DWR in December 2017 and January 2018. The vulnerability assessment uses a simplified slope geometry defined by crest elevation, crest width, landside levee height, waterside and landside slope, landside berm height and slope, if present, and landside toe-ditch location, if present. The specified vertical accuracy of the 2017 LiDAR was reported as 0.65 feet, but with better vertical accuracy of 0.33 feet in non-vegetated areas (e.g., levee crown). 2007 LiDAR collected by DWR was the source of past geometry studies, such as the Delta Levee Investment Strategy (Arcadis, 2017). The 2007 LiDAR data was also incorporated into this levee vulnerability study by comparing changes in crest elevation between 2007 and 2017.

The levee vulnerability study was performed using 5,048 cross-sections developed along levee systems that fall within the NOP corridor. The cross-sections are typically spaced every 500 feet along the levee centerline. Internal levees that are not intended to provide flood protection are typically not included in the data set. A summary of the cross-sections for each levee system included in the data set is presented below in Table 1.

Table 1. Levee Systems Considered in Vulnerability Assessment

Number of analysis sections considered per levee system

Levee System	Analysis Sections <sup>a</sup>		Levee System	Analysis Sections <sup>a</sup>
Bacon Island	152		McCormack-Williamson Tract	92
Bouldin Island	190		McDonald Island	145
Brack Tract	139		Medford Island	63
Byron Tract	138		Middle Roberts Island <sup>a</sup>	46
Canal Ranch	102		New Hope Tract	197
Coney Island	59		Pearson District	150
Dead Horse Island	28		Quimby Island	74
Drexler <sup>a</sup>	117		Randall Island	20
DWR Maintenance Area 9	91		Rindge Tract	166
Ehrheardt Club	80		Stark Tract	54
Fabian Tract	199		Staten Island	268
Glanville Tract	135		Terminous Tract	223
Grand Island	304		Tyler Island	242
Honker Lake Tract <sup>a</sup>	63		Union Island East	160
King Island	96		Union Island West	172
Libby McNeil	49		Upper Andrus Island	119
Lower Jones Tract	96		Venice Island	131
Upper Jones Tract	100		Victoria Island	159
Lower Roberts Island <sup>a</sup>	154		Walnut Grove	30
Mandeville Island	151		Woodward Island	94

<sup>&</sup>lt;sup>a</sup> Analysis sections are typically taken every 500 lineal feet.

#### 2.2 **Levee Geometry and Freeboard Standards**

Levee geometry standards and requirements in the Delta vary based on Project versus Non-Project levees, and Urban versus Non-Urban levees. Project levees are those levees that were either built, rebuilt, or adopted and maintained to USACE standards. Non-Project levees have been built and are maintained by private interests or local districts. Urban and Non-Urban levees are those that protect a population greater, or less than 10,000, respectively. The design standards that apply to the Delta levees are generally summarized in the following documents.

<sup>&</sup>lt;sup>b</sup> Lower Roberts Island, Middle Roberts Island, Upper Roberts Island (No data available), Honker Lake Tract and Drexler are all located within the same levee system. The data extents do not cover the entire perimeter of the levee system

## 2.2.1 Flood Hazard Mitigation Plan (HMP)

The Flood Hazard Mitigation Plan (HMP) prepared by DWR in 1983 outlined a set of conditions including maintenance and rehabilitation that a reclamation district should demonstrate in order to receive federal disaster relief. The plan included a "Short-Term Rehabilitation Plan" to receive federal disaster aid until a "Comprehensive Long-Term Mitigation Plan" is implemented. Part of the plan included the local agencies or district to maintain a minimum levee geometry consisting of:

- Freeboard of 1 foot above the 100-year WSE
- Crown width at least 16 feet
- Waterside slope inclinations of 1.5 horizontal to 1 vertical (H:V)
- Landside slope inclinations of 2H:1V
- Levees should have all-weather access roads

DWR and FEMA agreed to these design guidelines, and those reclamation districts that met the HMP qualified for FEMA disaster relief; however, this agreement is no longer in place.

### 2.2.2 Public Law 84-99

The Sacramento District of USACE established a Delta-specific standard for levees as part of PL 84-99, which defines the minimum levee configuration as:

- Freeboard of 1.5 feet over the 100-year WSE
- Crown width of 16 feet
- Waterside slope of 2H:1V
- Landside slope that ranges from 3H:1V to 5H:1V depending on height of levee and thickness of peat (See Attachment 1)

The minimum geometry criteria (USACE, 1987) was not intended to become a "design standard" for the Non-Project levees, but rather a uniform procedure to establish eligibility for PL 84-99 aid; however, the criteria are widely used and applied to both Non-Urban and Urban levees in the Delta.

### 2.2.3 DWR Bulletin 192-82

A levee investigation program undertaken as a joint study between DWR and USACE resulted in Bulletin 192-82, Delta Levees Investigation, by DWR (DWR, 1982) with supplemental geometry criteria later provided by DWR (DWR, 1989). The objective of the study was to primarily identify a plan to improve Non-Project levees within the Delta. Separate geometry standards were developed for levees protecting Urban and Agricultural (Non-Urban) Tracts. The levees evaluated in this levee vulnerability assessment classify as agricultural (Non-Urban) levees. The plan resulted in a minimum design configuration consisting of:

- Freeboard using a 300-year WSE should be 1.5 feet for agricultural (non-urban) areas and 3.0 feet for urban areas, respectively.
- Crown width at least 16 feet:
  - Waterside slope inclinations of 2H:1V (horizontal: vertical)
- Landside geometry varies based on height of levee, thickness of peat, and if a berm is present:
  - Landside slopes without a berm range from 3H:1H to 7H:1V (See Attachment 1)
  - Landside levee slopes with a berm are 3H:1V and include berms that are ½ the levee height with slopes that range from 3H:1V to 13H:1V (See Attachment 1)

The plan was intended to eventually have all levees within the Delta, regardless of being Urban or Non-Urban, upgraded to a minimum configuration and thus reducing the chances for failure. At the time of the Bulletin 192-82 report, this amounted to approximately 537 miles of Non-Project levees.

# 2.3 Relative Levee Vulnerability Criteria

The relative levee vulnerability criteria presented herein was developed internally by the DCA team and through feedback on the approach provided by the DCO. The criteria used in this TM to evaluate relative levee vulnerability are as follows:

- Criterion 1 Levees meeting levee geometry standards
- Criterion 2 Freeboard against the 100-year flood elevation
- Criterion 3 Ditches Proximity of toe-ditch (if present) to landside toe of levee or berm
- Criterion 4 Vulnerability to sea level rise
- Criterion 5 Change in Levee Crest Elevation between 2007 and 2017 LiDAR

Each criterion was evaluated using a rating score that varied from 1 to 4 scale (1 being unfavorable, 4 being favorable) and was assigned an importance (weighting) factor ranging from 1 to 5 scale (1 being of little importance, 5 being very important). The rating scores and importance factors were multiplied together for each criterion and the cumulative sum of all criteria provides a levee vulnerability score. The vulnerability scores can then be grouped and compared to provide a relative levee vulnerability rating (Levee Vulnerability Rating). Further discussion of the rating score for each criterion, levee vulnerability score, and levee vulnerability rating are provided below. A summary of the criteria, rating scores, and importance factors used in the assessment is provided in Table 2.

**Table 2. Levee Vulnerability Evaluation Criteria and Vulnerability Ratings** 

The criteria and rating scores used to evaluate levee cross-sections

	Importance	Rating Score (Lower Numbers = Worse Conditions)									
Criterion	Factor	1	2	3	4						
Levees Meeting Geometry Standards	5	Does Not Meet HMP	Meets HMP	Meets 84-99	Meets 192-82						
Freeboard Against 100- year Flood Elevation	3	less than 0 feet	0 to less than 1 foot	1 to less than 1.5 feet	Greater than or equal to 1.5 feet						
Toe Ditches	2	Ditch is present within 1 levee height from landside levee toe and no berm is present	Ditch is present within 1 to 2 levee heights from landside levee toe or 1 levee height from berm toe	Ditch is present within 2 to 4 levee heights from landside levee toe or 1 to 2 levee heights from berm toe	No ditch or ditch is present beyond 4 levee heights from landside levee toe and beyond 2 levee heights from berm toe						
Vulnerability to Sea Level Rise	2	Freeboard less than 0 feet	Freeboard 0 to less than 1 foot	Freeboard 1 to less than 1.5 feet	Freeboard greater than or equal to 1.5 feet						
Change in Levee Crest Elevation	2	>1.0 ft decrease in crest elevation from 2007 to 2017	0.75 to 1.0 ft decrease in crest elevation from 2007 to 2017	0.5 to 0.75 ft decrease in crest elevation from 2007 to 2017	Less than 0.5 ft decrease in crest elevation from 2007 to 2017						

The vulnerability criteria used in these analyses are based on LiDAR data and are therefore limited by the level of accuracy associated with the source data set. Allowable tolerances in the assessment were incorporated so that small deficiencies that are within the vertical accuracy tolerance of the source data do not flag a levee section as deficient. Allowable tolerances used for the criteria are provided below.

### 2.3.1 Criterion 1 – Levees Meeting Geometry Standards

In general terms, levee geometries defined by the HMP, PL84-99, and Bulletin 192-82 result in increasingly robust cross-sections; meaning that levees meeting Bulletin 192-82 also meet PL84-99 and HMP. For this analysis, the existing levee geometry was compared to each geometry standard provided in the HMP, PL84-99, and Bulletin 192-82; and a rating score was assigned to that levee section based on the most stringent standard met. Rating score metrics for this category were assigned as follows:

- Rating Score 1: Section does not meet a theoretical levee prism defined by HMP
- Rating Score 2: Section meets a theoretical levee prism defined by HMP
- Rating Score 3: Section meets a theoretical levee prism defined by PL84-99
- Rating Score 4: Section meets a theoretical levee prism defined by Bulletin 192-82

An allowable tolerance of +/- 0.1 feet was applied to levee crown elevations based on greater accuracy of LiDAR obtained in unvegetated areas. An allowable tolerance of +/- 1.0 feet was applied to levee toe elevations since the vertical accuracy of LiDAR data in vegetated areas is lower and levee performance is not as sensitive to geometry discrepancies compared to the levee crest.

Levee geometry standards are well known in the Delta and are commonly used as a metric to assess the robustness of a levee and its ability to provide flood protection. The geometry standards incorporate various minimum levee configurations that provide a uniform landside slope stability factor of safety of 1.25 for PL84-99 (USACE, 1987) and a landside stability factor of safety of 1.3 for Bulletin 192-82 (DWR, 1982). Given the significance of levee geometry in evaluating the condition of Delta levees, an importance factor of 5 was selected for this criterion representing an overall weighting of approximately 36 percent of the levee vulnerability score.

## 2.3.2 Criterion 2 – Freeboard Against 100-year Flood Elevation

Freeboard is a metric that is embedded in geometry standards described above. Instances where levees meet PL84-99 or Bulletin 192-82 will also meet freeboard requirements. However, levees that meet freeboard requirements but do not meet a specific geometry standard still reduce flood potential relative to levees without adequate freeboard. Therefore, a freeboard assessment independent of meeting geometry standard is included in the relative vulnerability evaluation. The criterion scoring metrics were assigned as follows:

- Rating Score 1: Freeboard against the 100-year WSE is less than 0 feet.
- Rating Score 2: Freeboard against the 100-year WSE is greater than or equal to 0 feet but less than 1 foot.
- Rating Score 3: Freeboard against the 100-year WSE is greater than or equal to 1 foot but less than 1.5 feet.
- Rating Score 4: Freeboard against the 100-year WSE is greater than or equal to 1.5 feet.

Similar to Criterion 1, an allowable tolerance of +/- 0.1 feet was applied to levee crown elevations based on greater accuracy of LiDAR obtained in unvegetated areas.

Freeboard is directly linked to crest elevation, which represents the maximum flood elevation a levee can withstand prior to overtopping. Levees can be tall, but not meet a levee geometry standard due to slope inclinations, and still provide a higher level of flood protection compared to a levee with a lower crest elevation. An importance factor of 3 was selected for this criterion, which represents an overall weighting of approximately 22 percent of the levee vulnerability score.

### 2.3.3 Criterion 3 – Toe-Ditches

Ditches in the Delta are often required to manage ground water levels and distribute irrigation water within a reclamation district. However, a consequence of installing toe-ditches is they can reduce the overall stability of a levee and create potentially adverse seepage paths below the levee. The presence of a toe-ditch does not mean the levee is unstable or will fail due to seepage, but it does increase the levee vulnerability relative to levees that do not have a toe-ditch. The effect of a toe-ditch on a levee is influenced by the proximity of the toe-ditch to the levee and depth of the toe-ditch. The toe-ditch depths are not well defined by the existing data sources and were therefore not considered in this assessment. The toe-ditch location relative to the landside levee toe or berm toe was considered. Rating scores for this category were assigned as follows:

- Rating Score 1: Ditch is present within 1 levee height from the landside levee toe and no berm is present.
- Rating Score 2: Ditch is present within 1 to 2 levee heights from the landside levee toe or 1 levee height from the berm toe.
- Rating Score 3: Ditch is present within 2 to 4 levee heights from the landside levee toe or 1 to 2 levee height from the berm toe.
- Rating Score 4: No ditch is present, or the ditch is present beyond 4 levee heights from the landside levee toe, or beyond 2 levee heights from the berm toe.

The presence of a toe-ditch will typically have a negative impact on levee stability and protection against seepage and piping. However, they can also be necessary to manage groundwater and can be safely implemented if properly designed. An importance factor of 2 was selected for this criterion, which represents an overall weighting of approximately 14 percent of the levee vulnerability score.

## 2.3.4 Criterion 4 – Vulnerability to Sea Level Rise

The conceptual construction timeline for the Project will occur over a roughly 10-year span between about 2030 and 2040 and has potential to be impacted by SLR. A uniform increase of 1.5 feet was added to the 100-year WSE for this levee vulnerability study as a proxy for site-specific hydraulic studies to capture potential SLR as well as changes in future hydraulic conditions and uncertainty. The 1.5-foot increase in WSE was added uniformly to the 100-year WSE and was redefined as 100-year SLR WSE for this levee vulnerability study. Freeboard was then checked against the 100-year SLR WSE. Rating scores for this category were assigned as follows:

- Rating Score 1: Freeboard against the 100-year SLR WSE less than 0 feet.
- Rating Score 2: Freeboard against the 100-year SLR WSE is greater than or equal to 0 feet but less than 1 foot.
- Rating Score 3: Freeboard against the 100-year SLR WSE is greater than or equal to 1 foot but less than 1.5 feet.
- Rating Score 4: Freeboard against the 100-year SLR WSE is greater than or equal to 1.5 feet.

Similar to Criterion 1 and 2, an allowable tolerance of +/- 0.1 feet was applied to levee crown elevations based on greater accuracy of LiDAR obtained in unvegetated areas.

The vulnerability to SLR criterion considers a levee system's resiliency to increasing flood elevations during future periods of potential Project construction. The uniform increase of 1.5 feet is not based on site-specific hydraulic analyses and would generally be considered conservative for most Delta levees during a likely period of construction through 2040, particularly those along the eastern margins of the Delta. An importance factor of 2 was selected for this criterion, which represents an overall weighting of approximately 14 percent of the levee vulnerability score.

### 2.3.5 Criterion 5 – Change in Levee Crest Elevation

Levees within the Delta are often underlain by thick deposits of highly compressible soils that are constantly settling due to consolidation. As a result, levees that currently meet a particular geometry standard may not meet standards in the future due to settlement in combination with rising water levels. Reclamation districts also implement periodic upgrades that may include levee crest raises or other modifications to the levee geometry to maintain compliance with levee standards. Changes in crest elevation may be influenced by both ongoing settlement and/or modifications to the levee cross-section. Crest elevation decreases between the 2007 and 2017 LiDAR would be most likely attributed to settlement and provide an indication of how the levee crest may change over time and in turn lead to future flood risks. Rating scores for this category were assigned as follows:

- Rating Score 1: Crest elevation decrease greater than 1.0 feet between the 2007 and 2017 LiDAR
- Rating Score 2: Crest elevation decrease equal to 0.75 feet to less than 1.0 feet between the 2007 and 2017 LiDAR
- Rating Score 3: Crest elevation decrease equal to 0.5 feet to less than 0.75 feet between the 2007 and 2017 LiDAR
- Rating Score 4: Crest elevation decrease less than 0.5 feet between the 2007 and 2017 LiDAR.

Decreases in levee crest elevation over time do not provide a direct metric of levee flood resilience but rather an indication of settlement that may be occurring. Ongoing settlement may be connected to soft compressible foundations as well as a lack of continual attention to maintain the levee geometry. In both cases, decreases in levee crest elevation reflect a condition that may increase flooding risk during a future construction period. An importance factor of 2 was selected for this criterion, which represents an overall weighting of approximately 14 percent of the levee vulnerability score.

### 2.3.6 Relative Levee Vulnerability Rating

The rating scores and importance factors for the relative levee vulnerability criteria were multiplied together and summed to provide a levee vulnerability score that ranged from 14 to 56 for each levee cross-section. The lowest possible vulnerability score (highest relative vulnerability) a cross-section could receive is 14 which results from receiving a rating score of one for each of the five criteria. Similarly, the highest possible vulnerability score (lowest relative vulnerability) a cross-section could receive is 56 which is obtained by receiving a rating score of four for each of the five criteria.

The levee vulnerability scores provide a single metric that can be used to compare the relative vulnerability of one levee cross-section to another. By combining the levee vulnerability scores for all cross-sections and binning the results into quartiles, relative vulnerability ratings (Levee Rating) were developed. Each data quartile contains about ¼ of the resulting levee vulnerability scores and were

assigned a Levee Rating of "Very Low," "Low," "Medium," or "High" relative vulnerability. The levee scores associated with each Levee Rating are:

- High relative vulnerability: levee vulnerability score range is from 14 to 38
- Medium relative vulnerability: levee vulnerability score range is from 39 to 47
- Low relative vulnerability: levee vulnerability score range is from 48 to 52
- Very Low relative vulnerability: levee vulnerability score range is from 53 to 56

#### 2.4 **Assumptions and Limitations**

The levee vulnerability assessments provided herein are based on available topographic data, subsurface data (peat/organics thickness), and existing WSEs (100-yr and 300-yr). The results of the analyses are therefore influenced by the accuracy of available data as discussed above. Assumptions used to perform the relative vulnerability assessment include the following:

- The simplified cross-section consisting of points at the waterside contact between the slope and river level, levee crest hinges, levee toe, berm toe (if present), and ditch hinges (if present) adequately represents existing levee geometry for the purposes of this study.
- Source topography is based on LiDAR and does not include bathymetry. The LiDAR data set used for this vulnerability assessment does not define the waterside toe elevation or waterside slope below the waterline at the time of the survey. These unknowns are acknowledged and assumed to be negligible.
- Inaccuracies and uncertainty present within source data will affect sections similarly throughout the Delta.
- Seismic performance and deformation were not explicitly considered in the levee vulnerability evaluations. However, some factors considered in the levee geometry evaluations are applicable to seismic vulnerability, such as peat thickness as an input in determining levee slope targets.

#### 3. **Analysis and Results**

The analyses were performed using the criteria and assumptions presented in Section 2 Methodology. Results are summarized in the following subsections.

#### 3.1 **Levee Geometry Standards**

The results of the levee geometry standards evaluation are summarized as a percentage of cross-sections within a levee system that received a rating score of 1, 2, 3 or 4 as shown below in Table 3. A graphical presentation of the results is presented in Figures 3-1a and 3-1b (See Attachment 2).

**Table 3. Levee Geometry Standards Summary of Results** 

Percentage of levee system sections resulting in each Standard Levee Geometry criterion rating

	Percentage of Levee Sections in Data Bin <sup>a</sup>					Percentage of Levee Sections Data Bin <sup>a</sup>				
Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> <sup>e</sup>		Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> <sup>e</sup>
Bacon Island	4	39	39	18	McCo	ormack-Williamson Tract	90	7	2	1
Bouldin Island	0	42	41	17	McDo	onald Island	0	1	1	99
Brack Tract	17	58	21	4	Medf	ford Island	0	37	29	35
Byron Tract	0	0	1	99	Midd	lle Roberts Island	87	0	0	13
Canal Ranch	25	39	23	13	New	Hope Tract	12	18	23	47
Coney Island	0	8	25	66	Pears	son District	0	0	0	100
Dead Horse Island	61	36	4	0	Quim	nby Island	1	4	34	61
Drexler	54	0	4	42	Rand	all Island	0	0	0	100
DWR Maintenance Area 9	2	0	0	98	Rinda	ge Tract	1	11	43	46
Ehrheardt Club	41	3	3	54	Stark	Tract	0	0	0	100
Fabian Tract	0	0	0	100	State	en Island	1	43	27	28
Glanville Tract	27	17	18	38	Term	inous Tract	25	48	13	14
Grand Island	0	0	1	99	Tyler	Island	6	31	28	36
Honker Lake Tract	56	8	5	32	Unio	n Island East	0	0	0	100
King Island	0	14	44	43	Unio	n Island West	0	0	13	87
Libby McNeil	10	6	6	78	Uppe	er Andrus Island	0	2	18	80
Lower Jones Tract	1	5	22	72	Venic	ce Island	6	69	14	11
Upper Jones Tract	10	28	17	45	Victo	ria Island	0	1	22	77
Lower Roberts Island	0	6	8	86	Waln	ut Grove	0	47	17	37
Mandeville Island	0	23	30	47	Woo	dward Island	0	15	12	73

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each rating score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> Rating Score 1: Does not meet minimum Hazard Mitigation Plan (HMP) geometry

<sup>&</sup>lt;sup>c</sup> Rating Score 2: Meets HMP geometry

<sup>&</sup>lt;sup>d</sup> Rating Score 3: Meets PL84-99 geometry

<sup>&</sup>lt;sup>e</sup> Rating Score 4: Meets Bulletin 192-82 geometry

## 3.2 Freeboard

The results of the freeboard evaluation are summarized as a percentage of cross-sections within a levee system that received a rating score of 1, 2, 3 or 4 as shown below in Table 4. A graphical presentation of the results is presented in Figures 3-2a and 3-2b (See Attachment 2).

**Table 4. Freeboard Summary of Results** 

Percentage of levee system sections resulting in each Freeboard criterion rating

	Percentage of Levee Sections Data Bin <sup>a</sup>				Percentage of Sections Da	
Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> <sup>e</sup>	Levee System 1 <sup>b</sup> 2 <sup>c</sup> 3	<b>3</b> <sup>d</sup> <b>4</b> <sup>e</sup>
Bacon Island	0	1	16	82	McCormack-Williamson Tract 53 36	5 5
Bouldin Island	0	0	8	92	McDonald Island 0 0	0 100
Brack Tract	2	6	8	84	Medford Island 0 0	0 100
Byron Tract	0	0	0	100	Middle Roberts Island 0 0	0 100
Canal Ranch	2	18	25	56	New Hope Tract 4 2 1	.1 84
Coney Island	0	0	0	100	Pearson District 0 0	0 100
Dead Horse Island	4	50	29	18	Quimby Island 0 1	0 99
Drexler	40	6	0	54	Randall Island 0 0	0 100
DWR Maintenance Area 9	2	0	0	98	Rindge Tract 1 0	0 99
Ehrheardt Club	36	0	0	64	Stark Tract 0 0	0 100
Fabian Tract	0	0	0	100	Staten Island 0 0	6 93
Glanville Tract	13	11	2	73	Terminous Tract 11 10 2	26 54
Grand Island	0	0	0	100	Tyler Island 3 3 1	.1 83
Honker Lake Tract	13	13	25	50	Union Island East 0 0	0 100
King Island	0	0	2	98	Union Island West 0 0	0 100
Libby McNeil	2	0	0	98	Upper Andrus Island 0 0	2 98
Lower Jones Tract	0	0	2	98	Venice Island 0 6 1	.8 76
Upper Jones Tract	0	7	5	88	Victoria Island 0 0	0 100
Lower Roberts Island	0	0	1	99	Walnut Grove 0 0	0 100
Mandeville Island	0	0	5	95	Woodward Island 0 0	0 100

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each rating score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> Rating Score 1: Freeboard against the 100-year WSE less than 0 feet

<sup>&</sup>lt;sup>c</sup> Rating Score 2: Freeboard against the 100-year WSE is greater than or equal to 0 feet but less than 1 foot

<sup>&</sup>lt;sup>d</sup> Rating Score 3: Freeboard against the 100-year WSE is greater than or equal to 1 foot but less than 1.5 feet

e Rating Score 4: Freeboard against the 100-year WSE is greater than or equal to 1.5 feet

#### 3.3 **Toe Ditch Proximity**

The results of the toe-ditch evaluation are summarized as a percentage of cross-sections within a levee system that received a rating score of 1, 2, 3 or 4 as shown below in Table 5. A graphical presentation of the results is presented in Figures 3-3a and 3-3b (See Attachment 2).

**Table 5. Toe Ditch Proximity Summary of Results** Percentage of levee system sections resulting in each Landside Toe Ditch criterion rating

	Percentage of Levee Sections Data Bin <sup>a</sup>				Percentage of Levee Sections Data Bin <sup>a</sup>					
Levee System	<b>1</b> b	<b>2</b> <sup>c</sup>	<b>3</b> <sup>d</sup>	<b>4</b> e	Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	<b>3</b> <sup>d</sup>	<b>4</b> e	
Bacon Island	0	63	13	25	McCormack-Williamson Tract	0	68	0	32	
Bouldin Island	0	40	14	46	McDonald Island	0	70	0	30	
Brack Tract	12	67	1	21	Medford Island	0	90	0	10	
Byron Tract	20	58	0	22	Middle Roberts Island	24	0	0	76	
Canal Ranch	0	71	1	28	New Hope Tract	7	32	1	61	
Coney Island	0	90	0	10	Pearson District	16	1	1	82	
Dead Horse Island	0	0	0	100	Quimby Island	0	93	0	7	
Drexler	51	25	0	24	Randall Island	0	0	0	100	
DWR Maintenance Area 9	0	2	0	98	Rindge Tract	0	74	1	25	
Ehrheardt Club	6	15	0	79	Stark Tract	37	13	2	48	
Fabian Tract	24	30	1	46	Staten Island	1	90	0	8	
Glanville Tract	34	16	7	42	Terminous Tract	16	54	1	29	
Grand Island	1	7	0	92	Tyler Island	12	37	0	52	
Honker Lake Tract	0	0	0	100	Union Island East	31	35	0	34	
King Island	0	51	0	49	Union Island West	3	65	2	31	
Libby McNeil	0	4	4	92	Upper Andrus Island	5	3	1	92	
Lower Jones Tract	0	69	0	31	Venice Island	0	72	3	25	
Upper Jones Tract	2	59	1	38	Victoria Island	2	87	1	11	
Lower Roberts Island	0	0	0	100	Walnut Grove	0	50	0	50	
Mandeville Island	0	72	1	27	Woodward Island	0	84	2	14	

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each rating score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> Rating Score 1: Toe ditch is present within 1 levee height form the landside levee toe and no berm is present

<sup>&</sup>lt;sup>c</sup> Rating Score 2: Toe ditch is present within 1 to 2 levee heights from the landside levee toe or 1 levee height from the berm toe

<sup>&</sup>lt;sup>d</sup> Rating Score 3: Toe ditch is present within 2 to 4 levee heights from the landside levee toe or 1 to 2 levee height from the berm toe

<sup>&</sup>lt;sup>e</sup> Rating Score 4: No toe-ditch is present, or the ditch is present beyond 4 levee heights from the landside levee toe and beyond 2 levee height from the berm toe

# 3.4 Vulnerability to Sea Level Rise

The results of the SLR evaluation are summarized as a percentage of cross-sections within a levee system that received a rating score of 1, 2, 3 or 4 as shown below in Table 6. A graphical presentation of the results is presented in Figures 3-4a and 3-4b (See Attachment 2).

**Table 6. Sea Level Rise Freeboard Summary of Results** 

Percentage of levee system sections resulting in each Sea Level Rise criterion score

	Percentage of Levee Sections Data Bin <sup>a</sup>				Percentage of Levee Sections Data Bin <sup>a</sup>					
Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> e	Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> <sup>e</sup>	
Bacon Island	18	78	1	3	McCormack-Williamson Tract	95	3	0	2	
Bouldin Island	8	81	8	3	McDonald Island	0	8	30	62	
Brack Tract	16	81	1	2	Medford Island	0	76	19	5	
Byron Tract	0	0	0	100	Middle Roberts Island	87	0	0	13	
Canal Ranch	44	47	6	3	New Hope Tract	16	31	10	43	
Coney Island	0	36	8	56	Pearson District	0	0	0	100	
Dead Horse Island	82	18	0	0	Quimby Island	1	14	15	70	
Drexler	54	5	7	34	Randall Island	0	0	0	100	
DWR Maintenance Area 9	2	0	0	98	Rindge Tract	1	92	4	4	
Ehrheardt Club	36	10	0	54	Stark Tract	0	0	0	100	
Fabian Tract	0	0	0	100	Staten Island	7	55	17	21	
Glanville Tract	27	30	10	34	Terminous Tract	46	43	5	6	
Grand Island	0	0	0	100	Tyler Island	17	34	22	27	
Honker Lake Tract	62	11	2	25	Union Island East	0	0	0	100	
King Island	2	91	5	2	Union Island West	0	0	0	100	
Libby McNeil	2	4	4	90	Upper Andrus Island	2	24	6	69	
Lower Jones Tract	2	52	28	18	Venice Island	24	64	8	4	
Upper Jones Tract	12	50	20	18	Victoria Island	0	1	11	88	
Lower Roberts Island	1	7	17	75	Walnut Grove	0	0	0	100	
Mandeville Island	5	75	17	3	Woodward Island	0	13	47	40	

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each rating score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> Rating Score 1: Freeboard against the 100-year SLR WSE less than 0 feet

<sup>&</sup>lt;sup>c</sup> Rating Score 2: Freeboard against the 100-year SLR WSE is greater than or equal to 0 feet but less than 1 foot

<sup>&</sup>lt;sup>d</sup> Rating Score 3: Freeboard against the 100-year SLR WSE is greater than or equal to 1 foot but less than 1.5 feet

<sup>&</sup>lt;sup>e</sup> Rating Score 4: Freeboard against the 100-year SLR WSE is greater than or equal to 1.5 feet.

# 3.5 Change in Levee Crest Elevation from 2007 to 2017

The results of the change in levee crest elevation are summarized as a percentage of cross-sections within a levee system that received a rating score of 1, 2, 3 or 4 as shown below in Table 7. A graphical presentation of the results is presented in Figures 3-5a and 3-5b (See Attachment 2).

Table 7. Change in Crest Elevation from 2007 to 2017 Summary of Results

Percentage of levee system sections resulting in each crest elevation change criterion score

	Percentage of Levee Sections Data Bin <sup>a</sup>				Percentage of Levee Sections Data Bin <sup>a</sup>				
Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> e	Levee System	<b>1</b> <sup>b</sup>	<b>2</b> <sup>c</sup>	3 <sup>d</sup>	<b>4</b> e
Bacon Island	4	4	13	80	McCormack-Williamson Tract	0	0	0	100
Bouldin Island	2	6	12	81	McDonald Island	1	0	1	97
Brack Tract	2	4	12	83	Medford Island	29	32	21	19
Byron Tract	16	1	1	83	Middle Roberts Island	7	0	4	89
Canal Ranch	0	0	0	100	New Hope Tract	3	5	11	82
Coney Island	10	14	5	71	Pearson District	0	0	0	100
Dead Horse Island	0	0	0	100	Quimby Island	3	1	3	93
Drexler	15	2	4	79	Randall Island	0	0	0	100
DWR Maintenance Area 9	0	0	0	100	Rindge Tract	8	17	31	43
Ehrheardt Club	1	0	0	99	Stark Tract	2	0	0	98
Fabian Tract	0	0	2	98	Staten Island	0	0	3	97
Glanville Tract	0	0	0	100	Terminous Tract	21	6	9	64
Grand Island	0	0	0	100	Tyler Island	5	3	3	89
Honker Lake Tract	5	3	13	79	Union Island East	0	0	1	99
King Island	7	6	21	66	Union Island West	0	1	3	96
Libby McNeil	0	0	0	100	Upper Andrus Island	0	0	0	100
Lower Jones Tract	13	8	15	65	Venice Island	27	14	11	47
Upper Jones Tract	39	6	13	42	Victoria Island	27	9	9	55
Lower Roberts Island	13	7	11	69	Walnut Grove	0	0	0	100
Mandeville Island	23	11	9	58	Woodward Island	22	16	13	49

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each rating score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> Rating Score 1: Crest elevation decrease greater than 1.0 feet between the 2007 and 2017 LiDAR

 $<sup>^{\</sup>rm c}$  Rating Score 2: Crest elevation decrease equal to 0.75 feet to less than 1.0 feet between the 2007 and 2017 LiDAR

<sup>&</sup>lt;sup>d</sup> Rating Score 3: Crest elevation decrease equal to 0.5 feet to less than 0.75 feet between the 2007 and 2017 LiDAR

e Rating Score 4: Crest elevation decrease less than 0.5 feet between the 2007 and 2017 LiDAR

# 3.6 Relative Levee Vulnerability Rating

The results of the relative levee vulnerability rating evaluation are summarized as a percentage of cross-sections within a levee system that received a Levee Rating of Very Low, Low, Medium, or High relative vulnerability as shown below in Table 8. A graphical presentation of the results is presented in Figures 3-6a and 3-6b (See Attachment 2).

**Table 8. Relative Levee Vulnerability Rating of Results** 

Percentage of levee system sections resulting in each Relative Vulnerability Rating

	Percentage of Levee Sections Data Bin <sup>a</sup>					Percentage of Levee Sections Data Bin <sup>a</sup>				
Levee System	Нb	Mc	<b>L</b> d	<b>VL</b> e	Levee System H <sup>b</sup> M <sup>c</sup> L <sup>d</sup>	<b>VL</b> e				
Bacon Island	38	45	16	0	McCormack-Williamson Tract 95 4 1	0				
Bouldin Island	27	56	11	6	McDonald Island 1 1 72	27				
Brack Tract	70	27	1	1	Medford Island 43 46 11	0				
Byron Tract	0	14	67	19	Middle Roberts Island 87 0 0	13				
Canal Ranch	63	24	10	4	New Hope Tract 24 28 14	34				
Coney Island	12	29	54	5	Pearson District 0 0 17	83				
Dead Horse Island	86	14	0	0	Quimby Island 3 38 57	3				
Drexler	54	7	26	14	Randall Island 0 0 0	100				
DWR Maintenance Area 9	2	0	2	96	Rindge Tract 11 58 31	0				
Ehrheardt Club	43	4	16	38	Stark Tract         0         2         48	50				
Fabian Tract	0	0	54	46	Staten Island 35 36 26	2				
Glanville Tract	31	31	20	18	Terminous Tract 70 22 7	2				
Grand Island	0	0	9	91	Tyler Island 29 25 27	18				
Honker Lake Tract	62	10	25	3	Union Island East 0 0 66	34				
King Island	5	71	21	3	Union Island West 0 3 74	23				
Libby McNeil	2	16	8	73	Upper Andrus Island 2 15 15	68				
Lower Jones Tract	6	33	49	11	Venice Island 76 15 7	3				
Upper Jones Tract	41	18	34	7	Victoria Island 1 33 58	8				
Lower Roberts Island	3	11	18	68	Walnut Grove 0 60 3	37				
Mandeville Island	25	44	28	3	Woodward Island 13 27 54	6				

<sup>&</sup>lt;sup>a</sup> Results indicate the percentage of evaluated cross-sections that fall into each vulnerability score bin. Due to rounding, total for each levee system may not add to 100 percent.

<sup>&</sup>lt;sup>b</sup> High relative vulnerability: vulnerability score range is from 14 to 38

<sup>&</sup>lt;sup>c</sup> Medium relative vulnerability: vulnerability score range is from 39 to 47

<sup>&</sup>lt;sup>d</sup> Low relative vulnerability: vulnerability score is from 48 to 52

<sup>&</sup>lt;sup>e</sup> Very low relative vulnerability: vulnerability score range is from 53 to 56

4.

# **Observations and Conclusions**

This TM presents the results of a relative levee vulnerability assessment performed for the Project. The results of this TM are intended as a screening level assessment to identify potential vulnerabilities within the Delta levee systems, not to be interpreted as design-level analyses. However, it should also be noted that screening of existing levee geometry as a means for prioritizing levee upgrades is common practice within the Delta and supported by DWR Delta Special Projects program managed by the Delta Levees group within DWR. Key observations and conclusions from the assessment include:

- The relative vulnerability ratings of Very Low, Low, Medium or High relative vulnerability are a metric to compare one levee cross-section and system to another and the bins were assigned based on all of the analyses performed. Of the approximately 5,000 cross-sections evaluated, approximately one-fourth of the sections received the highest possible vulnerability score and half of the cross-sections received scores of over 48 or higher (Low vulnerability or better). As a result, levee cross-sections may meet relatively stringent current standards (i.e., PL 84-99 geometry and freeboard) but may still be characterized as having a "medium relative vulnerability" due to the presence of a toe-ditch or lack of future freeboard when considering potential impacts due to SLR.
- When siting project infrastructure, consider the relative vulnerability ratings and levee geometry standards as part of the selection siting process. Levee locations with higher vulnerability rankings may require more robust mitigations and/or repair footprints.
- Lower Roberts Island, Middle Roberts Island, Upper Roberts Island, Honker Lake Tract, and Drexler are separated internally by low/smaller internal levees which are not designed or intended to provide flood protection against a 100-year WSE assumed in this levee vulnerability study. These levee systems share a perimeter levee that provides flood protection. The LiDAR source data covers the western and northern extent of the shared perimeter levee but does not cover the eastern and southern extent. As a result, the assessment presented herein for this levee system is based on a partial data set within this portion of the Delta. It is also noted that Lower Roberts Island includes an interior levee along the southern boundary that separates it from the other levee systems and provides additional flood protection although not to a 100-year WSE standard.
- The results provided herein should be used in conjunction with sound engineering judgement when selecting the locations of project infrastructure. This analysis provides an indication of levee relative vulnerability at discrete cross-section locations. The higher relative vulnerability rankings serve as an indicator of levee locations within the tunnel corridors that may be deficient and require further evaluation and possible mitigation. Future repairs should consider type, magnitude and extents of deficiencies.
- As project components progress from feasibility and planning level studies to design level studies, obtaining site-specific subsurface data and testing and conducting site-specific engineering analyses will be needed.

# 5. References

Arcadis. 2017. Delta Levee Investment Strategy. Final Report.

California Department of Water Resources (DWR). 1983. Flood Hazard Mitigation Plan for the Sacramento-San Joaquin Delta.

California Department of Water Resources (DWR). 1982. Bulletin 192-82 Delta Levees Investigation.

California Department of Water Resources (DWR). 1989. Memorandum Report: Minimum Geometry Criteria for Rehabilitating Existing Levees in Sacramento-San Joaquin Delta.

California Department of Water Resources (DWR). 2011. Analysis of Delta Levees Compliance of HMP [Hazard Mitigation Plan] and PL 84-99 Design Geometry. Digital Elevation Model.

California Department of Water Resources (DWR). 2020. Notice of Preparation of Environmental Impact Report for the Delta Conveyance Project. January.

United States Army Corps of Engineers (USACE). 1976. Sacramento San Joaquin Delta California, Stage-Frequency Study, Hydrology.

United States Army Corps of Engineers (USACE). 1987. Guidelines for Rehabilitation of Non-Federal Levees in the Sacramento-San Joaquin Delta, CA.

United States Army Corps of Engineers (USACE). 1992. Office Report: Sacramento San Joaquin Delta California, Special Study, Hydrology.

URS Corporation (URS). 2008. Technical Memorandum: Delta Risk Management Strategy, Phase 1, Topical Area: Levee Vulnerability Final.

#### 6. **Document History and Quality Assurance**

Reviewers listed have completed an internal quality review check and approval process for deliverable documents that is consistent with procedures and directives identified by the Engineering Design Manager (EDM) and the DCA.

		Approval Names and Roles										
Date	Prepared by	Internal Quality Control review by	Consistency review by	Approved for submission by								
12/23/2021	Elliott Ticen / Project Engineer	Graham Bradner / DCA Executive Director Michael Conant / Senior Engineer	Gwen Buchholz / DCA Environmental Consultant	Terry Krause / EDM Project Manager								

This interim document is considered preliminary and was prepared under the responsible charge of Michael Conant, California Professional Engineering License C79228.

#### **Note to Reader**

This is an early foundational technical document. Contents therefore reflect the timeframe associated with submission of the initial and final drafts. Only minor editorial and document date revisions have been made to the current Conformed Final Draft for Administrative Draft Engineering Project Report version.

Attachment 1 Levee Geometry Standards

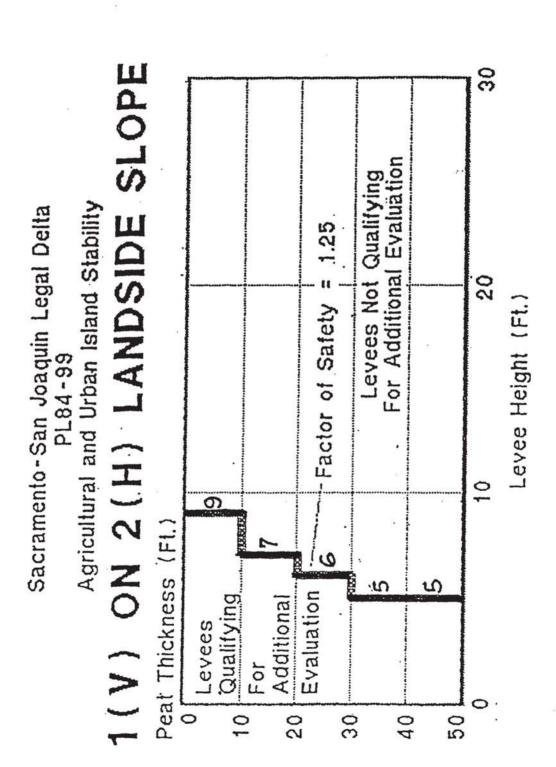
## **Attachment 1. Levee Geometry Standards**

Presentation of levee geometry standards relating peat/organics thickness and levee height to allowable landside levee or berm slopes for Public Law 84-99 (USACE, 1987) and DWR Bulletin 192-82 (DWR, 1982) and (DWR, 1989). Content excerpted from these standards includes:

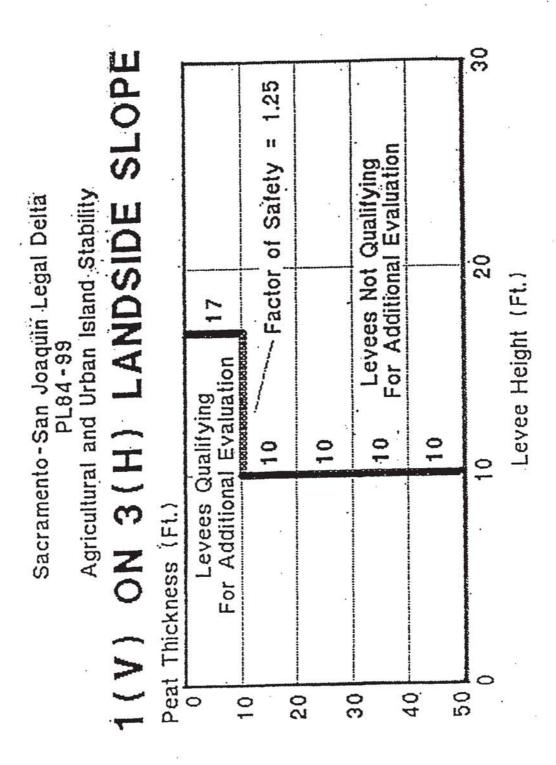
- Public Law 84-99 Four charts showing the required landside height versus peat thickness for a specific landside levee slope. Each chart is developed for a different landside slope which include 2H:1V, 3H:1V, 4H:1V, and 5H:1V. This geometry standards applies to both urban and non-urban levee systems.
- DWR Bulletin 192-82 Four charts which present the minimum landside slope or berm slope based on levee height, presence of berm, contours of peat thickness and land use. Figures 1 and 3 are for urban tracts and presented for completeness. Figures 2 and 4 present the reference standards for agricultural (non-urban) tracts which were used for the geometry assessment in this TM.



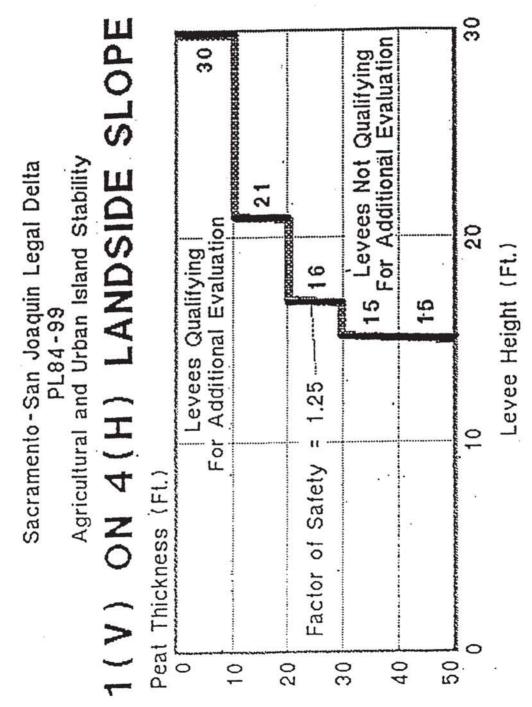
PL 84-99



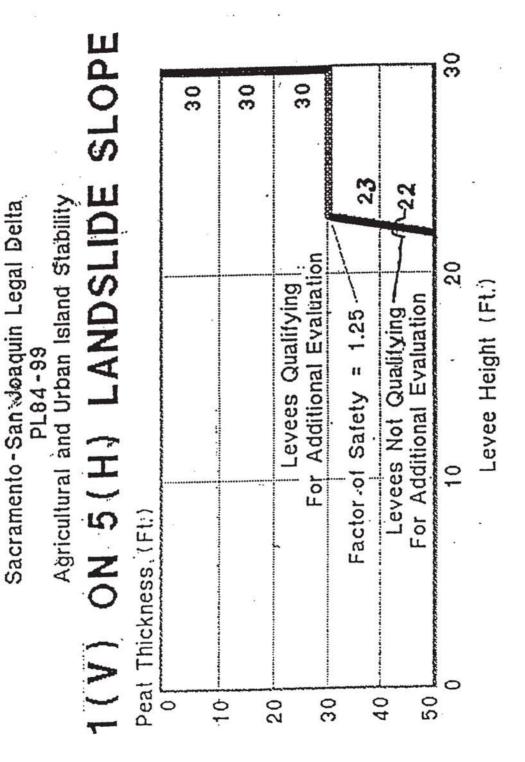
PL 84-99

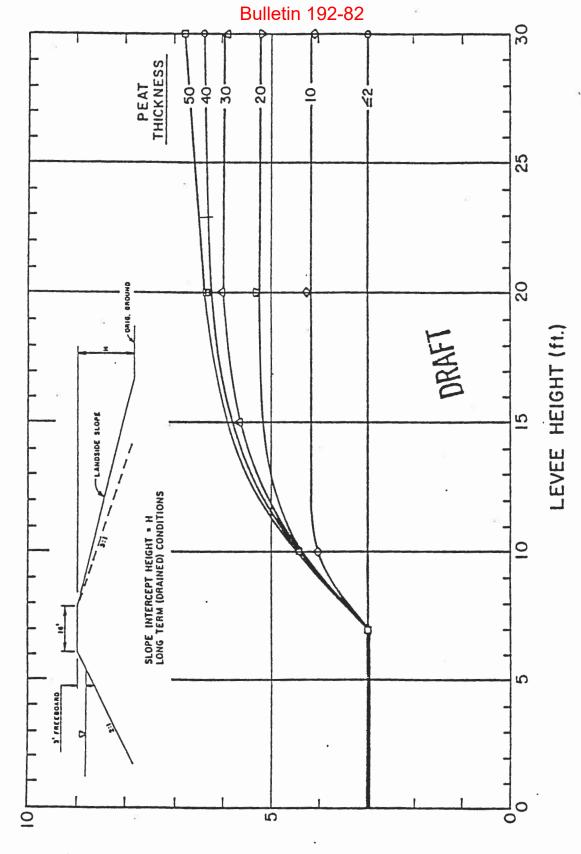


PL 84-99

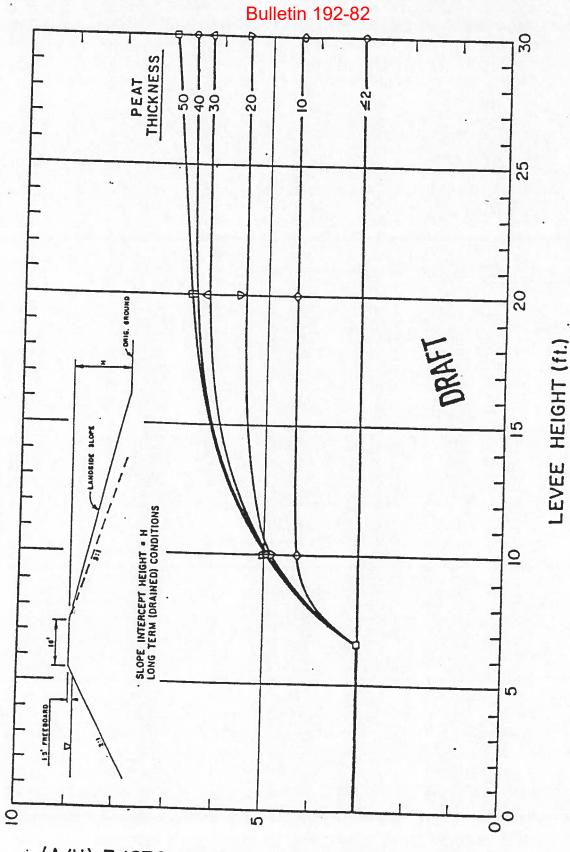


PL 84-99

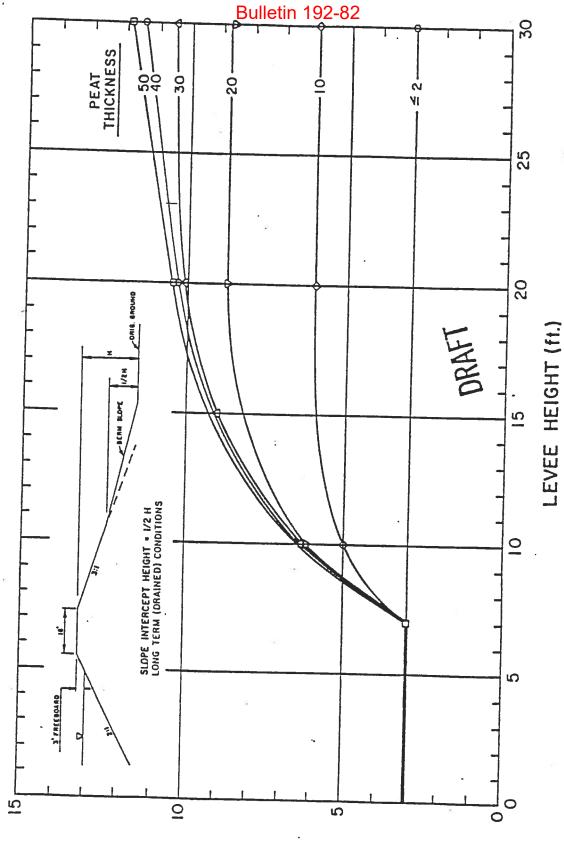




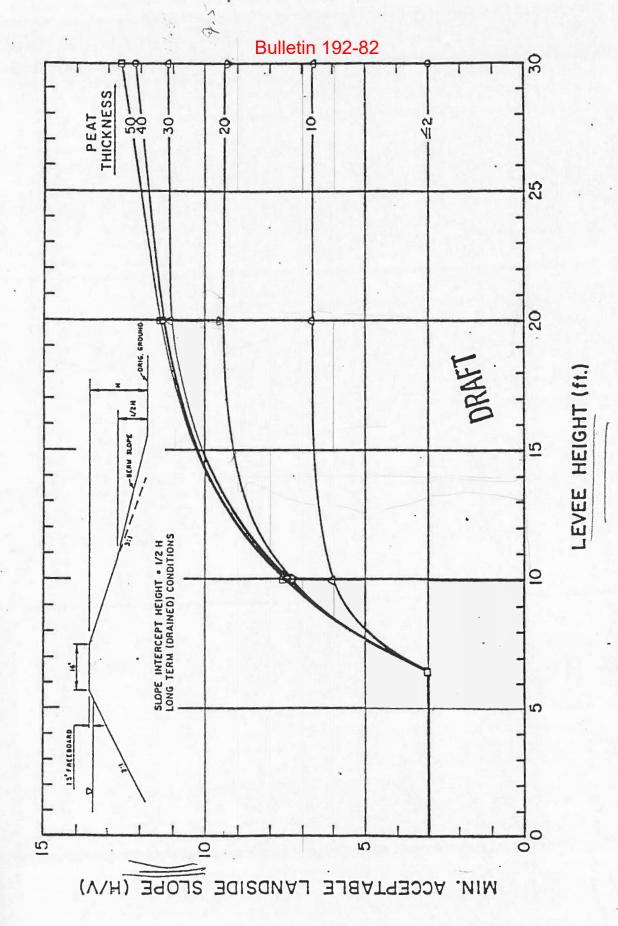
MIN. ACCEPTABLE LANDSIDE SLOPE (H/V)

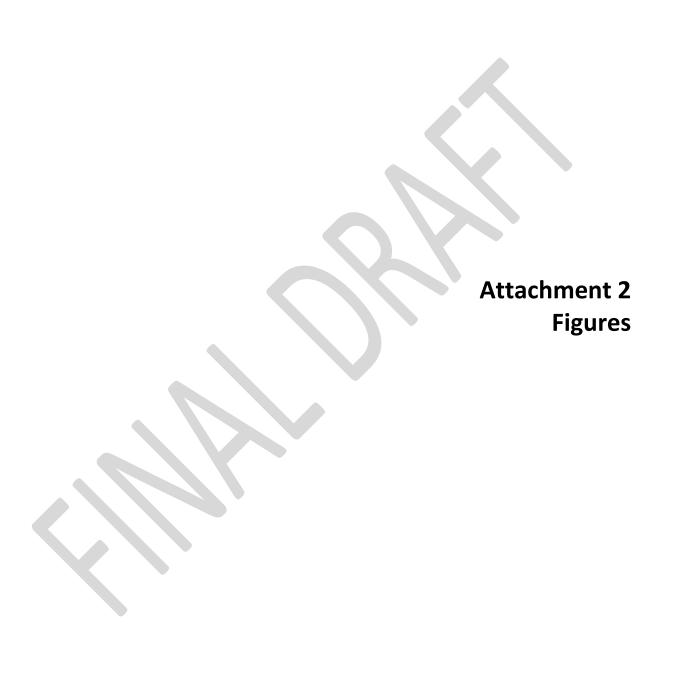


MIN. ACCEPTABLE LANDSIDE SLOPE (H/V)



MIN. ACCEPTABLE BERM SLOPE (H/V)





# Attachment 2. Figures

Graphical presentation of the relative levee vulnerability criteria and relative levee vulnerability results by cross-section location.



